

Corrections for  
Extended Finite Element Method,  
Wiley/Blackwell, 2008

Chapter 2  
Fracture Mechanics, A Review

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{yz} \\ \sigma_{zx} \end{Bmatrix} = \begin{bmatrix} \lambda + 2\mu & \lambda & \lambda & 0 & 0 & 0 \\ \lambda & \lambda + 2\mu & \lambda & 0 & 0 & 0 \\ \lambda & \lambda & \lambda + 2\mu & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix} \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{zz} \\ 2\varepsilon_{xy} \\ 2\varepsilon_{yz} \\ 2\varepsilon_{zx} \end{Bmatrix} \quad (2.7)$$

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \end{Bmatrix} \quad (2.10)$$

$$\sigma_{xx} = \frac{\partial^2 \Phi}{\partial y^2} \quad (2.19)$$

$$\sigma_{yy} = \frac{\partial^2 \Phi}{\partial x^2} \quad (2.20)$$

$$\nabla^4 \Phi = \nabla^2 (\sigma_{xx} + \sigma_{yy}) = 0 \quad (2.22)$$

$$-\sigma_{\alpha\alpha} + \sigma_{\beta\beta} + 2i\sigma_{\alpha\beta} = 2[\bar{z}\psi''(z) + \chi''(z)] e^{2i\theta} \quad (2.32)$$

$$\begin{cases} \sigma_{rr} = 0 \\ \sigma_{\theta\theta} = \sigma_0(1 + 2 \cos 2\theta) \\ \sigma_{r\theta} = 0 \end{cases} \quad (2.42)$$

$$u_z = \begin{cases} 0 & \text{plane strain} \\ -\frac{\nu z}{E}(\sigma_{xx} + \sigma_{yy}) & \text{plane stress} \end{cases} \quad (2.72)$$

$$\sigma_{xx} = -\frac{K_{II}}{\sqrt{2\pi r}} \sin \frac{\theta}{2} \left( 2 + \cos \frac{\theta}{2} \cos \frac{3\theta}{2} \right) \quad (2.73)$$

$$\sigma_{zz} = \begin{cases} \nu(\sigma_{xx} + \sigma_{yy}) & \text{plane strain} \\ 0 & \text{plane stress} \end{cases} \quad (2.76)$$

$$u_z = \begin{cases} 0 & \text{plane strain} \\ -\frac{\nu z}{E}(\sigma_{xx} + \sigma_{yy}) & \text{plane stress} \end{cases} \quad (2.80)$$

$$u_z = \frac{K_{III}}{\mu} \sqrt{\frac{r}{2\pi}} \sin \frac{\theta}{2} \quad (2.85)$$

$$\sigma_{r\theta} = \frac{K_{II}}{\sqrt{2\pi r}} \left( \frac{1}{4} \cos \frac{\theta}{2} + \frac{3}{4} \cos \frac{3\theta}{2} \right) \quad (2.91)$$

$$K_I = \left[ 1.12 - 0.23 \left( \frac{a}{2b} \right) + 10.56 \left( \frac{a}{2b} \right)^2 - 21.74 \left( \frac{a}{2b} \right)^3 + 30.42 \left( \frac{a}{2b} \right)^4 \right] \sigma_0 \sqrt{\pi a} \quad (2.93)$$

$$\frac{\partial^2(\Pi + U_I)}{\partial a^2} \begin{cases} < 0 & \text{unstable fracture} \\ > 0 & \text{stable fracture} \\ = 0 & \text{neutral equilibrium} \end{cases} \quad (2.119)$$

$$K_I = \sigma \cos^2 \theta_0 \sqrt{\pi a} \quad (2.124)$$

$$K_{II}(\theta) = g(\theta) \left( K_{II} \cos \theta - \frac{1}{2} K_I \sin \theta \right) \quad (2.133)$$

$$G(\theta) = \frac{1}{4E'} g^2(\theta) \left[ (1 + 3 \cos^2 \theta) K_I^2 + 8 \sin \theta \cos \theta K_I K_{II} + (9 - 5 \cos^2 \theta) K_{II}^2 \right] \quad (2.136)$$

--- (page 39)

Stress intensity factors can be determined at different radial distances from the crack tip by equating the numerically obtained displacements with their analytical expression in terms of the SIF. For plane **strain** problems in the  $xy$  plane (Fig. 2.10):

$$K_I = \mu \sqrt{\frac{2\pi}{r}} \frac{u_y^b - u_y^a}{4(1-\nu)} \quad (2.147)$$

$$K_{II} = \mu \sqrt{\frac{2\pi}{r}} \frac{u_x^b - u_x^a}{4(1-\nu)} \quad (2.148)$$

$$s = 2 \sqrt{\frac{G_1 - G_2}{K'}} \quad (2.163)$$

$$\mathbf{J} = \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{bmatrix}, \quad \mathbf{J}^{-1} = \frac{1}{\det \mathbf{J}} \begin{bmatrix} \frac{\partial y}{\partial \eta} & -\frac{\partial y}{\partial \xi} \\ -\frac{\partial x}{\partial \eta} & \frac{\partial x}{\partial \xi} \end{bmatrix} \quad (2.176)$$

$$\delta = r_p = \frac{1}{2\pi} \frac{K_I^2}{\sigma_{yld}^2} = \frac{a}{2} \left( \frac{\sigma_0}{\sigma_{yld}} \right)^2 \quad (2.189)$$

$$\sigma_n = \sigma_{xx} \cos^2 \theta + 2\sigma_{xy} \sin \theta \cos \theta + \sigma_{yy} \sin^2 \theta \quad (2.231)$$

$$\sigma_t = (\sigma_{yy} - \sigma_{xx}) \sin \theta \cos \theta + \sigma_{xy} (\cos^2 \theta - \sin^2 \theta) \quad (2.232)$$

$$u_t = -u_x \sin \theta + u_y \cos \theta \quad (2.234)$$

$$\begin{aligned} J = & \int w(-\sin \theta dx + \cos \theta dy) \\ & - \int [\sigma_{xx} \varepsilon_{xx} \cos \theta dy + \sigma_{xy} \varepsilon_{yy} \sin \theta dy - \sigma_{xy} \varepsilon_{xx} \cos \theta dx - \sigma_{yy} \varepsilon_{yy} \sin \theta dx \\ & + \sigma_{xy} \varepsilon_{xy} \cos \theta dy - \sigma_{xy} \varepsilon_{xy} \sin \theta dx + (\sigma_{xx} \sin \theta - \sigma_{xy} \cos \theta) du_n \\ & - (\sigma_{yy} \sin \theta + \sigma_{xy} \cos \theta) du_t] \end{aligned} \quad (2.235)$$

$$J = \int_{A^*} \left[ \sigma_{ij} \frac{\partial u_i}{\partial x_1} - W_s \delta_{1j} \right] \frac{\partial q}{\partial x_j} dA \quad (2.244)$$

$$J^{\text{act}} = \int_{A^*} \left[ \sigma_{ij} \frac{\partial u_i}{\partial x_1} - W_s \delta_{1j} \right] \frac{\partial q}{\partial x_j} dA \quad (2.248)$$

$$J^{\text{aux}} = \int_{A^*} \left[ \sigma_{ij}^{\text{aux}} \frac{\partial u_i^{\text{aux}}}{\partial x_1} - W^{\text{aux}} \delta_{1j} \right] \frac{\partial q}{\partial x_j} dA \quad (2.249)$$

$$M = \int_{A^*} \left[ \sigma_{ij} \frac{\partial u_i^{\text{aux}}}{\partial x_1} + \sigma_{ij}^{\text{aux}} \frac{\partial u_i}{\partial x_1} - W^{\text{M}} \delta_{1j} \right] \frac{\partial q}{\partial x_j} dA \quad (2.250)$$

$$W_s = \frac{1}{2} \sigma_{ij} \varepsilon_{ij} \quad (2.251)$$

$$W^{\text{aux}} = \frac{1}{2} \sigma_{ij}^{\text{aux}} \varepsilon_{ij}^{\text{aux}} \quad (2.252)$$

### Chapter 3

## Extended Finite Element Method for Isotropic Problems

$$\mathbf{J} = \begin{bmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} \end{bmatrix} \quad \mathbf{J}^{-1} = \frac{1}{\det \mathbf{J}} \begin{bmatrix} \frac{\partial y}{\partial \eta} & -\frac{\partial y}{\partial \xi} \\ -\frac{\partial x}{\partial \eta} & \frac{\partial x}{\partial \xi} \end{bmatrix} \quad (3.7)$$

$$J = \int_{A^*} \left[ \sigma_{ij} \frac{\partial u_i}{\partial x_j} - W_s \delta_{li} \right] \frac{\partial q}{\partial x_j} d\Gamma \quad (3.14)$$

$$J = \sum_{\substack{\text{elements} \\ \text{in } A^*}} \left( \sum_{g=1}^{ng} \left\{ \left[ \left( \sigma_{ij} \frac{\partial u_i}{\partial x_j} - W_s \delta_{li} \right) \frac{\partial q}{\partial x_j} \right] \det \left( \frac{\partial x_j}{\partial \xi_k} \right) \right\} W_g \right) \quad (3.15)$$

$$\mathbf{u}^h(\mathbf{x}_i) = \mathbf{u}_i + \sum_{k=1}^m p_k(\mathbf{x}_i) \mathbf{a}_{ik} \quad (3.34)$$

$$\delta(\xi) = \begin{cases} \frac{1}{2\beta} + \frac{1}{2\beta} \cos \frac{\pi\xi}{\beta} & -\beta < \xi < \beta \\ 0 & \text{otherwise} \end{cases} \quad (3.55)$$

$$\int_{\Omega} \boldsymbol{\sigma} \cdot \boldsymbol{\delta \varepsilon} d\Omega = \int_{\Omega} \mathbf{f}^b \cdot \boldsymbol{\delta \mathbf{u}} d\Omega + \int_{\Gamma} \mathbf{f}^t \cdot \boldsymbol{\delta \mathbf{u}} d\Gamma \quad (3.79)$$

$$\mathbf{B}_i^a = \begin{bmatrix} (N_i[H(\xi) - H(\xi_i)])_{,x} & 0 \\ 0 & (N_i[H(\xi) - H(\xi_i)])_{,y} \\ (N_i[H(\xi) - H(\xi_i)])_{,y} & (N_i[H(\xi) - H(\xi_i)])_{,x} \end{bmatrix} \quad (3.92)$$

$$H_{,i}(\xi) = \begin{cases} 1 & \text{at crack} \\ 0 & \text{otherwise} \end{cases} \quad (3.95)$$

## Chapter 4 Static Fracture Analysis of Composites

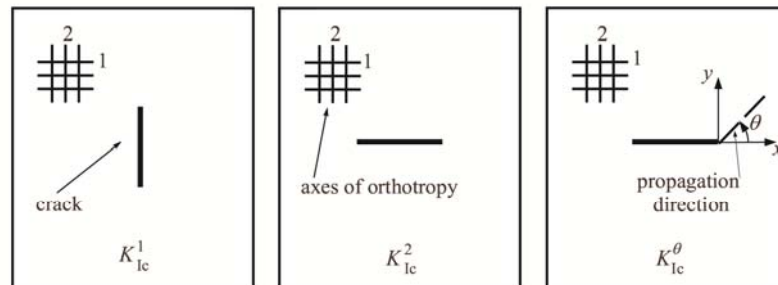


Figure 4.1 Fracture toughness for a homogeneous anisotropic solid.

$$\max \left\{ \frac{1}{\cos^2 \theta + \frac{K_{Ic}^1}{K_{Ic}^2} \sin^2 \theta} \left[ \operatorname{Re} \left[ \frac{s_1 t_1 - s_2 t_2}{s_1 - s_2} \right] + \frac{K_{II}}{K_I} \operatorname{Re} \left[ \frac{t_1 - t_2}{s_1 - s_2} \right] \right] \right\} \quad (4.27)$$

$$J = \int_{\Gamma} \left( W_s \delta_{1j} - \sigma_{ij} \frac{\partial u_i}{\partial x_1} \right) \mathbf{n}_j d\Gamma \quad (4.60)$$

$$J = \int_{\Gamma} \left( W_s \delta_{1j} - \sigma_{ij} \frac{\partial u_i}{\partial x_1} \right) \mathbf{n}_j d\Gamma \quad (4.121)$$

$$M^{(1)} = 2t_{11}K_I + t_{12}K_{II} \quad (4.134)$$

$$\begin{cases} G_I = -\frac{\pi}{2} K_I c_{22} \operatorname{Im} \left[ \frac{K_I (s_1 + s_2) + K_{II}}{s_1 s_2} \right] \\ G_{II} = \frac{\pi}{2} K_{II} c_{11} \operatorname{Im} [K_{II} (s_1 + s_2) + K_I s_1 s_2] \end{cases} \quad (4.29)$$

## Chapter 5

### XFEM for Cohesive Cracks

$$\mathbf{f}_t^c = \mathbf{f}_t^c(\delta \mathbf{u}_n, \delta \mathbf{u}_t) \quad (5.20)$$

**Figure 5.16** Equilibrium of a body with a cohesive crack.